

DRAFT Guidance Specification for Porous or Dense-Graded Hot-Mix Asphalt Pavement Structures for Storm Water Management

Revised November 29, 2006

INTRODUCTION

Storm water management (SWM) is a challenge facing our ever expanding communities. A porous or dense-graded hot-mix asphalt pavement structures offer an opportunity to address this challenge through parking lot and other paved area applications. Multi-purpose applications can also be utilized such as SWM with commercial, residential, and recreational facility design concepts. With proper design and installation, the system is designed to allow infiltration of the storm water runoff into the pavement structure, then release the treated water to replenish the groundwater, eliminating the need for a detention basin that often requires additional land. The system is comprised of a permeable or dense-graded hot-mix asphalt (HMA) surface placed over a granular working platform on top of a reservoir of large stone. The lower reservoir layer is designed to have the storage capacity to hold the water. If dense-graded HMA is used as the surface material instead of porous HMA, the system needs to be designed to allow storm water runoff to infiltrate into the reservoir layer (e.g. riverjack, draitiles, pipes, etc.) as determined by the engineer.

This document provides guidance and recommendations for specifying the storm water management system. More information is available through various reports including the National Asphalt Pavement Association Information Series 131 titled *Porous Asphalt Pavements*, and through websites including www.asphaltisbest.com and www.hotmix.org

SUBGRADE PREPARATION

- Existing subgrade under bed areas shall NOT be compacted or subject to excessive construction equipment traffic prior to geotextile and stone bed placement.
- Bring subgrade of stone infiltration bed to line, grade, and elevations indicated. Fill and lightly regrade any areas damaged by erosion, ponding, or traffic compaction before the placing of stone. All bed bottoms are level grade.

INFILTRATION BED INSTALLATION

- Place geotextile in accordance with manufacturer's standards and recommendations. Adjacent strips of geotextile shall overlap a minimum of sixteen inches (16"). Secure geotextile at least four feet (4') outside of bed and take any steps necessary to prevent any runoff or sediment from entering the storage bed.
- Install coarse aggregate in 8-inch maximum lifts. Lightly compact each layer with equipment, keeping equipment movement over storage bed subgrades to a minimum. Install aggregate to grades indicated on the drawings.
- Install choker base course (see Materials section) aggregate evenly over surface of stone bed, sufficient to allow placement of pavement, and notify Engineer for approval. Choker base course shall be sufficient to allow for even placement of hot-mix asphalt (HMA) but no thicker than nominal 1-inch in depth.
- Following placement of bed aggregate, the geotextile shall be folded back along all bed edges to protect from sediment washout along bed edges. At least a four-foot edge strip shall be used to protect beds from adjacent bare soil. This edge strip shall remain in place until all

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bare soils contiguous to beds are stabilized and vegetated. In addition, take any other necessary steps to prevent sediment from washing into beds during site development. When the site is fully stabilized, temporary sediment control devices shall be removed.

All aggregates (coarse and choker base aggregate materials) within infiltration beds shall meet the following:

- Maximum Wash Loss of 1.5 %
- The Los Angeles Rattler (LAR) loss on the coarse aggregate fraction (material retained on the 4.75 mm [No. 4] sieve) shall not exceed 40% for any individual source used within the mix. The composite maximum LAR loss shall not exceed 35.
- Material shall be 80% crushed (one fractured face)
- The use of recycled materials will not be permitted. Recycled materials shall include, but are not limited to: glass, recycled asphaltic pavement, crushed concrete, and roofing shingles.

Coarse aggregate for the groundwater infiltration beds shall be uniformly graded meeting modified AASHTO #2, #3, or #5:

Modified AASHTO size number 2*

U.S. Standard Sieve Size	Percent Passing
3" (75mm)	100
2 1/2" (63mm)	80-100
1 1/2" (37.5mm)	20-60
3/4" (19.0mm)	0-10

Modified AASHTO size number 3*

U.S. Standard Sieve Size	Percent Passing
2 1/2" (63mm)	100
2" (50mm)	70-100
1 1/2" (37.5mm)	35-70
1/2" (12.5mm)	0-5

Modified AASHTO size number 5*

U.S. Standard Sieve Size	Percent Passing
1 1/2" (37.5mm)	100
1" (25mm)	100
1/2" (12.5mm)	0-15
#4 (4.75mm)	0-5

*As an Alternate, a uniformly graded gradation with a minimum void space of 40% is allowed according to AASHTO T19.

Compaction of the coarse aggregate for the groundwater infiltration beds shall be by **ordinary compaction**. One pass with vibration is allowable (low amplitude, high frequency) for aggregate interlock. **Continuous vibratory passes are not allowed.**

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Choker base course aggregate for groundwater infiltration beds shall have the following gradation (AASHTO size number 57):

U.S. Standard Sieve Size	Percent Passing
1 ½" (37.5mm)	100
1" (25mm)	95-100
½" (12.5mm)	25-60
4 (4.75mm)	0-10
8 (2.36mm)	0-5

Non-woven geotextile shall be Mirafi 160N, or approved equal.

DENSE-GRADED HOT-MIX ASPHALT

Modify MnDOT (2360) Plant Mixed Asphalt Pavement Combined 2360/2350 (Gyratory/Marshall Design) Specification obtained from the following MnDOT Bituminous Office web page <http://www.mrr.dot.state.mn.us/pavement/bituminous/bituminous.asp>

Mineral aggregate shall meet aggregate size A or 4 (1/2" maximum size – LV4) gradation of modified MnDOT 2350/2360 for the type and thickness of mixture specified. As an alternate, aggregate size B or 3 (3/4" maximum size) may be substituted.

Bituminous material shall be PG (Performance Grade) 58-28 (non-wear virgin and recycle non-wear or wear) or 64-22 for all virgin mix wearing. Design asphalt content by weight shall be in accordance to MnDOT 2360 (Table 2360.2E, Table 2360.3-B2a and Table 2360.32b).

POROUS HOT-MIX ASPHALT

Replace 2360.1 DESCRIPTION with:

This work consists of the construction of a Porous Hot Mix Asphalt Wearing Course Mixture (Porous HMA). The work shall be in accordance with these specifications and shall conform to the lines, grades, thicknesses, and typical cross-sections shown on the plans or established by the Engineer.

Modify 2360.2E Gradation Requirement as follows:

**Table 2360.2-E
Aggregate Gradation Broad Bands
(% passing of total washed gradation)**

Sieve Size, mm (inch)	Porous HMA
19.0 (¾")	100
12.5 (½")	85-100
9.5 (3/8")	55-75
4.75 (#4)	10-25
2.36 (#8)	5-10
0.075 (#200)	2-4

Replace 2360.2C1 (Los Angeles Rattler [LAR] Test) with:

The Los Angeles Rattler loss on the coarse aggregate fraction (material retained on the 4.75-mm [#4] sieve) shall not exceed 35 percent for any individual source used within the mix.

Replace 2360.2C2 (Soundness) with:

The magnesium sulfate soundness requirements shall only apply to Class D aggregate (including the Class D portion of combination materials) containing more than 20 percent non-igneous particles by mass. If the wearing course aggregate contains non-igneous particle in excess of 20 percent but less than or equal to 45 percent by mass, the total non-igneous fraction shall not show a magnesium sulfate soundness loss of more than 20 percent. If the wearing course aggregates contain non-igneous particles in excess of 45 percent, by mass, the total non-igneous fraction shall not show a magnesium sulfate soundness loss of more than 10 percent.

Replace 2360.2C3 (Insoluble Residue) with:

If Class B carbonate is used in the mix, the minus 75- μ m sieve size portion of the insoluble residue shall not exceed 10 percent.

Replace 2360.2F Additives, 2360.2F1 Mineral Filler, 2360.2F2 Hydrated Lime with:

An additive is any material added to an asphalt mixture or material, such as mineral filler, asphalt additives, anti-strip, stabilizers, and similar products that do not have a specific pay item. When a Contract requires additives, compensation is included with the pay items for the appropriate mixture. If the Engineer directs the Contractor to incorporate additives, the compensation will be as Extra Work, at the unit price specified in the proposal. The Contracting Authority will not compensate the Contractor for additives incorporated at the Contractor's option.

Additives shall not be incorporated into the mixture without approval of the Engineer. Anti-foaming agents shall be added to asphalt cement at the manufacturer's recommended dosage rate.

B1 Mineral Filler AASHTO M17

B1a Composition

Mineral filler shall consist of carbonate dust, Portland cement, hydrated lime, crushed rock screening, or rotary limekiln dust.

Crushed rock screenings to be used as mineral filler shall be of such composition and quality that the bituminous mixture containing the rock screenings will have stability and durability equivalent to those of the comparable mixture containing one of the other acceptable filler materials. The rock screenings shall be free from clay and shale.

Mineral filler prepared from rock dust, slag dust, and similar materials shall be free from organic impurities and have a plasticity index not greater than 4 (AASHTO T 90).

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B1a1 Hydrated Lime

Hydrated lime used in asphalt mixtures shall meet the requirements of ASTM C977 and have a maximum of eight percent unhydrated oxides (as received basis).

B1b Gradation

Mineral filler shall be graded within the following limits:

Percent finer than 0.600mm (#30).....	100
Percent finer than 0.300mm (#50).....	95-100
Percent finer than 0.075mm (#200).....	70-100

B1c Condition

Mineral filler that is to be added directly to the dried aggregate for the bituminous mixture shall be thoroughly dry and free from lumps consisting of aggregations of fine particles.

Crushed rock screenings used as mineral filler shall be of uniform gradation and shall be processed and handled un such a manner as will prevent segregation. The rock screenings shall be dried by passing through the dryer.

B1d Sampling and Testing

- 1) Sampling.....MnDOT Bituminous Manual
- 2) Fineness
Sieve Analysis.....AASHTO T 27
Hydrometer Analysis.....AASHTO T 88
(A) This procedure is modified to permit the use of Gum Arabic as a dispersing agent if flocculation occurs.
- 3) Plasticity Index.....AASHTO T 89 & 90
- 4) Specific Gravity.....AASHTO T100

Insert: 2360.2 F5

F5 Porous HMA Asphalt Stabilizer.....AASHTO MP8-02

An asphalt stabilizer additive may be used to control drain-down. The stabilizer additive shall be a cellulose fiber conforming to AASHTO MP8-02.

Stabilizer dosage rate shall be within 0.2-0.4 percent by weight of the total mix.

Insert 2360.2G

G Asphalt Binder Material AASHTO M 320

The asphalt binder material shall be PG 58-28 or PG 64-22.

2360.3 MIXTURE DESIGN

A Mixture Design General

Include:

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- a) No paving will be allowed without a Contractor or consultant laboratory job mix formula design using Gyrotory (50 gyrations) mix design. The Contractor shall provide all data necessary (aggregate and mixture) and pay all costs.
- b) All mixture shall be designated as wearing course.
- c) Additional information can be found in Appendix A of NAPA's Porous Asphalt Pavements, Quality Improvement Series 131, however these specs shall take precedence.

Insert B1a Desired Aggregate Blend

Prior to the start of asphalt production, the Contractor shall provide:

1. Dry-rodded voids in coarse aggregate of the coarse aggregate fraction (VCA_{DRC}). Coarse aggregate is defined as the aggregate fraction retained on the 4.75-mm (#4) sieve.
 - a. Compact coarse aggregate according to AASHTO T19
 - b. Calculate VCA_{DRC}

$$VCA_{DRC} = \frac{G_{CA}\gamma_w - \gamma_s}{G_{CA}\gamma_w} \times 100$$

Where,

G_{CA} = bulk specific gravity of the coarse aggregate (AASHTO T85)

γ_s = unit weight of the coarse aggregate fraction in the dry-rodded condition (AASHTO T19)

γ_w = unit weight of water

2. Voids in the coarse aggregate of the mix (VCA_{MIX}) according to the following steps.
 - a. For each trial gradation prepare three batches at between 6.0 and 6.5 percent asphalt binder. Include fibers if used.
 - b. Compact two specimens from each trial gradation using 50 gyrations of the Superpave gyratory compactor.
 - i. Determine the bulk specific gravity (G_{mb}^*) of each specimen by Corelok (ASTM D6752) or Calculation (AASHTO T 269 or ASTM 3203) following the method stated in section 6.2 to determine the density of the compacted specimen.
 - ii. Determine the VCA_{MIX} of each compacted specimen.

$$VCA_{MIX} = 100 - \left[\frac{G_{mb}}{G_{CA}} \times P_{CA} \right]$$

Where:

G_{CA} = bulk specific gravity of the coarse aggregate

G_{mb} = bulk specific gravity of compacted specimens

P_{CA} = percent coarse aggregate in the total mixture

- c. Use the remaining sample from each trial gradation to determine the theoretical maximum specific gravity (G_{mm}) of each trial
- d. To select design gradation, choose the trial gradation with the $VCA_{MIX} < VCA_{DRC}$ with high air voids.

Modify Table 2360.3-B2a as follows:

**Table 2360.3-B2a
Mixture Aggregate Requirements**

Aggregate Blend Property	Porous HMA
Coarse Aggregate Angularity (ASTM D5821) (one face), %- Wear & Non-Wear	55
Coarse Aggregate Absorption, % AASHTO T85, Mn/DOT modified	≤ 2
Voids in Coarse Aggregate (VCA_{drc}) AASHTO T19	VCA _{mix} < VCA _{drc}
Flat and Elongated Particles, max⁽²⁾ % by weight, (ASTM D 4791)	≤ 5 (5:1 ratio) ≤ 20 (2:1 ratio)
Clay Content⁽²⁾ (AASHTO T 176)	30
Total Spall in fraction retained on the 4.75mm (#4) sieve	2.5
Maximum Spall Content in Total Sample	2.5
Maximum Percent Lumps in fraction retained on the 4.75mm (#4) sieve	0.5
<u>Gyratory</u> Max. allowable RAP percentage Wear /Non Wear	30/30

Modify Table 2360.3-B2b as follows:

**Table 2360.3-B2b
Mixture Requirements**

Gyratory Mixture Requirements	Porous HMA
Gyrations for N _{initial}	N/A
Gyrations for N _{design}	50
Gyrations for N _{maximum}	N/A
Air Voids, % -- Wear	≥ 18%
Tensile Strength Ratio ⁽¹⁾ , min% at mix design	≥ 80
Fines/Effective Asphalt	≤ 1.2
Draindown - based on a 1 hour reading at the anticipated production temperature	≤ 0.3 %
Stabilizer by weight of total mix, %	0.2 – 0.4
VCA Ratio	VCA _{MIX} < VCA _{DRC}

(1) See 2360.4 E9. Use 150mm (6-inch) specimens for gyratory.

Insert: 2360.3 B2d Minimum Asphalt Content

The guideline for percent asphalt binder shall be 5.5 to 6.5 percent by weight of mix.

Delete Tables 2360.3-B2c (VMA Mixture Requirements) and 2360.3-B3 (Option B Mixture Requirements).

Modify 2360.3C(10)(a)

Using the selected design gradation, prepare mixes at the three binder contents in increments of 0.5 percent. Conduct draindown test (AASHTO T305 or ASTM D6390) on loose mix at a temperature 15°C higher than anticipated production temperature. Compact mix using 50 gyrations of a Superpave gyratory compactor and determine air void contents.

Modify 2360.3C(10)(g)

Test final mix for moisture susceptibility using the modified Lottman method (AASHTO T283). Retained tensile strength (TSR) ≥ 80 percent.

Insert: 2360.3C Documentation, Additional Documentation for Gyratory Design:

(G1a) For the trial blend(s), determine the Voids in the Coarse Aggregate-Dry Rodded Condition (VCA_{DRC}) according to AASHTO T19. The VCA ratio (VCA_{mix}/VCA_{DRC}) shall be less than 1.0, i.e. $VCA_{mix} < VCA_{DRC}$.

Modify 2360.3 C Documentation, under Additional Documentation for Gyratory Design, subsection (G2) with:

(G2) The design number of gyrations at N_{design} .

Replace 2360.3 C Documentation, under Additional Documentation for Gyratory Design, subsection (G4) with:

(G4) Evidence that the completed mixture will conform to all specified physical requirements as follows: asphalt content and densification % G_{mm} at N_{design} , design air voids (V_a), TSR, Fines/Effective Asphalt, draindown, percent Stabilizer by weight of total mix, and VCA ratio.

Insert under: 2360.3 C Documentation, Additional Documentation for Gyratory Design:

(G6) Percent and manufacturer's data for type of stabilizer used.

Replace the 2nd paragraph of 2360.4 D Sampling and Testing as follows:

The tests for mixture properties shall be conducted on representative portions of the mix, quartered from a larger sample of mixture taken from the truck box. The mixture shall be sampled from the truck box at the plant site after the truck has been fully loaded. The procedure for truck box sampling is as follows:

- 1) In at least three random locations in the truck box and at least 1 foot away from the sides take equal increment samples that when combined will form a field sample whose quantity equals or exceeds the minimum required.
- 2) Specifier personnel may monitor all samples obtained from the truck box.
- 3) An alternate sampling location may be recommended by the Contractor. Due to mixture coarseness, sampling from the uncompacted mat behind the paver is discouraged. Proper sampling of mixture from a windrow, prior to mixture entry to the paver, is recommended.

Modify Table 2360.4-D as follows:

**Table 2360.4-D
Production Testing Rates**

Production Test	Testing Rates	Test Reference	Spec Section
Bulk Specific Gravity	1 test per 450 metric tons (500 tons)	ASTM D6752 Mn/DOT modified	2360.4E2a
Maximum Specific Gravity	1 test per 450 metric tons (500 tons)	AASHTO T209 MnDOT modified	2360.4E3
Air Voids (calculated)	1 test per 450 metric tons (500 tons)	AASHTO T269, T312	2360.4E4
Asphalt Content	1 test per 450 metric tons (500 tons)	Bit & Lab Manual	2360.4E1
Gradation	1 test per 900 metric tons (1000 tons)	AASHTO T11 & T27	2360.4E6
Coarse Aggregate Angularity	1 test per 900 metric tons (1000 tons)	ASTM D5821	2360.4E7
VCA Ratio (calculation)	1 test per 450 metric tons (500 tons)	AASHTO T19	2360.4E14
Draindown	1 test per 450 metric tons (500 tons)	AASHTO T305 or ASTM D6390	2360.3

Delete Table 2360.4-E as follows:

Delete 2360.4E2

Insert: 2360.4 E13 Draindown Test.....AASHTO T305

Draindown that exceeds 0.3% is unacceptable. The Contractor shall take appropriate action to bring draindown values into specification. This action may include checking to determine if the stabilizer is being added, if the rate of adding stabilizer is correct and checking if the mixing temperature is excessive. The draindown test shall be performed at the production plant mixing temperature.

Insert: 2360.4 E14

Void in Coarse Aggregate (VCA) Ratio Calculation AASHTO PP41-02

To select gradation, choose the gradation with the $VCA_{MIX} < VCA_{DRC}$ with high air voids.

Modify 2360.4F(2a) Documentation (Records) to include:

- 15. If obtained, verification sample.
- 16. VCA ratio
- 17. Draindown value in %

Modify 2360.4 G to include:

- (6) VCA ratio
- (7) Draindown value in %

Replace Table 2360.4-H with:

**Table 2360.4-H
JMF Limits (N=4)**

Item	JMF Limits
% G_{mm} @ N_{design} / Production Air Void	±1.0
Asphalt Binder Content, Percent	±0.4
Sieve - % Passing*	
19, 12.5, 9.5 mm (3/4", 1/2", 3/8")	± 4
4.75, 2.36 mm (No. 4, No. 8)	± 3
0.075 mm (No. 200)	± 2.0

* JMF target values are not allowed outside the broadband requirements in Table 2360.2-E.

2360.4E 10 Modify Title:

**Aggregate Specific Gravity (G_{sb}) and Aggregate AbsorptionAASHTO T84 and T85
MnDOT modified**

Modify 2360.5 B Restrictions as follows:

Placement of Porous HMA mixtures will be allowed only when the ambient air temperatures are at least 10C (50F).

Modify 2360.5C1a(7) with:

Storage of the mix will be allowed during production at the risk of the Contractor and not to exceed 90 minutes.

Modify 2360.5C2a with:

HMA shall be placed with a track paver.

Modify 2360.5C2b with:

The HMA shall be covered.

Insert 2360.5D1

A uniform lower rate of application may be 0.01 – 0.02 gallons per square yard (undiluted) or 0.02 – 0.04 gallons per square yard (diluted).

Insert 2360.5F Construction Joints

For multiple lift construction, the Engineer and Contractor may elect to mutually agree to criss-cross the previously placed strip (lift) prior to placement operations.

Replace 2360.6 Pavement Density, with:

All compaction shall be by the Ordinary Compaction Method. Compaction of the hot-mix asphalt shall take place when the surface is cool enough to resist a 10-ton steel-wheeled roller [vibratory mode not allowed]. One or two passes is all that is required for proper compaction. More rolling could cause a reduction in the surface porosity which is unacceptable. Vibratory rollers and/or pneumatic-tired rollers are not allowed.

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C4 Trench Rollers

Trench rollers shall be self propelled and have a mass of not less than 2,960 pounds per foot of width.

C5 Mixture Temperature Controls

Unless directed by the Engineer in writing, no paving is allowed under the Ordinary Compaction Method when the air temperature is below 55°F when measured away from artificial heat. The minimum laydown temperature of the bituminous mix shall be 275 degrees Fahrenheit.

Other Guidance on Ordinary Compaction of HMA

Transporting of mix to the site shall be in vehicles with smooth, clean dump beds that have been sprayed with a non-petroleum release agent.

The porous bituminous courses shall be laid directly over the storage bed and stone base course to the specified finished thickness.

After final rolling, no vehicular traffic of any kind shall be permitted on the surface until cooling and hardening has taken place, and in no case within the first 24 hours.

Transition to adjacent impervious bituminous paving shall be merged neatly with flush, clean line. Finished paving shall be even, without pockets, and graded to elevations shown on drawing.

Porous pavement beds shall not be used for equipment or materials storage during construction, and under no circumstances shall vehicles be allowed to deposit soil on paved porous surfaces.

Establish and maintain required lines and elevations. The Engineer shall be notified for review and approval of final stake lines for the work before construction work is to begin. Finished surfaces shall be true to grade and even, free of roller marks and free of low spots to form puddles. All areas must drain.